

This is Chapter 5 “Thermo-chemo-mechanical shotcrete model” from my PhD thesis

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Enjoy reading.

Best regards,
Alexander Kluckner



Dipl.-Ing. Alexander Kluckner, BSc

**Tunnelling at greater depths:
Study on the ground and system behaviour
when passing a stiff rock block in a weak zone**

DOCTORAL THESIS

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Doktor der technischen Wissenschaften

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Chapter 5

Thermo-chemo-mechanical shotcrete model

At the *Semmering Base Tunnel* (SBT), which is under construction, geotechnical engineers use the software suite TUNNEL:Monitor ([142]) to analyse recorded displacements of the tunnel lining. Besides standard evaluation approaches (e.g., displacement vector analysis, state line diagrams) (for details, refer to [291, Chapter 5, p. 42ff]), the suite also allows to calculate strains, stresses, and utilisations within the lining from displacement data. The latter is possible because of developments by *Prof. Christian Hellmich* (Vienna University of Technology, Vienna, Austria) and his colleagues (cf., e.g., [154, 236]). The developments resulted in the so-called *hybrid method*. [302] details one of the latest versions of the method. Descriptions below are limited to the version implemented in TUNNEL:Monitor installed at the SBT and utilised for this thesis (cf. Chapter 8 on p. 169, Chapter 9 on p. 201, and Chapter 10 on p. 243). Note that this implementation does not make use of the hierarchical modelling technique described in [302, Section 2.3, p. 229f].

The hybrid method comprises two main operations (cf. [210, p. 1048]):

1. Reconstruction of the displacement field history from the displacements recorded at monitoring cross sections at the construction site and approximation of the strain field.
2. Application of thermal, chemical, and mechanical considerations to model the material behaviour of the shotcrete. The resulting concept constitutes the so-called *thermo-chemo-mechanical shotcrete model*.

The following sections give some information on the two operations. If not referenced, cited information bases on personal communications with the software engineers of TUNNEL:Monitor.

Once the strain field is available, applying the shotcrete model results in lining stresses and utilisations variable in both space and time (in incremental form). Six layers discretize the lining across its thickness. Stresses are calculated for each layer. The utilisation is the average across the lining thickness. The model considers only circumferential and longitudinal axial stresses. Because shotcrete linings usually are in a biaxial stress state (cf. Section 4.4 on p. 49), this simplification is reasonable (cf. [210, p. 1050]).

The implementation makes use of the Boltzmann superposition principle (cf. Footnote 6 on p. 206). According to [354], the application of this principle yields acceptable results for the viscoelastic system response of a given stress history as long as the load or imposed deformation, either causing compressive or tensile stresses, continuously increases. It does not in case the load or imposed deformation varies strongly in its direction (e.g., large temperature increase under

a restrained condition with compressive stresses first, then decrease of temperature causing a significant reduction of compressive stresses or even switch to tensile stresses). This excludes the stress decrease due to relaxation, provided that the load or imposed deformation still increases or at least remains unchanged. If the implemented shotcrete model uses Boltzmann's superposition principle without distinguishing between loading and unloading, it can therefore be assumed that it still yields acceptable results for stress histories primarily with an increase of load or imposed deformation over time.

5.1 Displacement and strain field

In the direction of the tunnel drive, monitoring cross sections are several metres apart from each other, each comprising a few monitoring targets only being uniformly distributed along the lining circumference (cf. Fig. 5.1 below and Fig. 8.5 on p. 180). To get a continuous displacement field in-plane (i.e., in the circumferential direction between monitoring targets), out-of-plane (i.e., in the longitudinal direction between neighbouring monitoring cross sections), and between two subsequent recordings regarding time, spatial and temporal interpolations are required. [158, p. 2107ff]

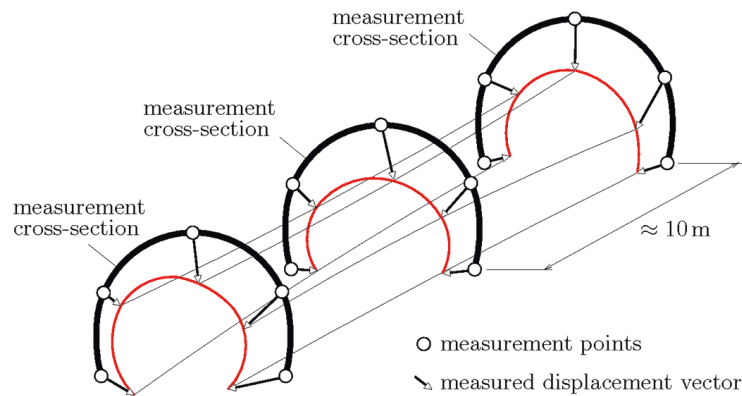


Figure 5.1: Conventional tunnel drives: Spatial interpolation of measured displacements in the circumferential direction between neighbouring measurement points (or targets), and in the longitudinal direction between neighbouring measurement (or monitoring) cross sections (from [302, Fig. 1, p. 228]).

First, interpolations are performed for each monitoring cross section using the in-plane displacement components. Spline functions allow for the temporal and spatial interpolations. The former depends on the number of measurement epochs available. Second, linear interpolations between two neighbouring monitoring cross sections result in the spatial displacement field. Longitudinal (or out-of-plane) displacements and out-of-plane bending remain unconsidered.

The approach works only in the scope of geometric linearity requiring deformations to be small compared to the dimensions of the engineering structure (cf. [242, p. 154]). If the hybrid method shall be able to simulate lining failure properly, it needs to consider geometric nonlinearity and surfaces of discontinuity. [154, p. 6]

Processing the changes in the displacement field (i.e., length of spline segments between calculation points, curvature at calculation points) yields the axial lining strain and the lining thickness. Plane cross sections being perpendicular to the circumferential lining axis before the deformation remain perpendicular during the deformation (fundamental assumption of the *Euler-Bernoulli* beam theory; cf., e.g., [334, Fig. D.9, p. 563]). This assumption is valid as long as the thickness is small.

5.2 Shotcrete model

The shotcrete model implemented in TUNNEL:Monitor at the SBT adopts the theory of thermodynamics of a chemically reactive porous continua by [86] considering the principle of mass conservation. Based on that theory, already in 1995, [401] developed a constitutive law for early age concrete. Here, the formation of the calcium silicate hydrate (C-S-H) phases on the microlevel is the main determinant of the description of the material behaviour (cf. [154, p. 6]). The approach decomposes the investigated continuum into representative elementary volumes (REV) and examines the energy state of each finite REV affected by chemo-physical transformations taking place inside (cf. [86] in [154, p. 8]). [399, 402] introduced chemoplasticity to consider for cracking. And [154] extended the concept towards shotcrete. Mechanical equilibrium and thermodynamics constitute the physics ([158, p. 2104]).

Shotcrete is a chemically reactive material with an evolving microstructure and each REV of the hardening shotcrete is the superposition of porous continua with n fluid phases and 1 skeleton phase (cf. Fig. 5.2). The latter comprises aggregates, unhydrated cement, and hydrates. Because hydrates already formed enclose unhydrated cement, free water (not yet combined with cement) needs to bypass those hydrates. The water has to diffuse through them. As the hydrates grow, it takes more time for the water to reach the unhydrated cement. This diffusion process determines the rate of hydration (cf. Section 4.1 on p. 47). [154, p. 6ff]

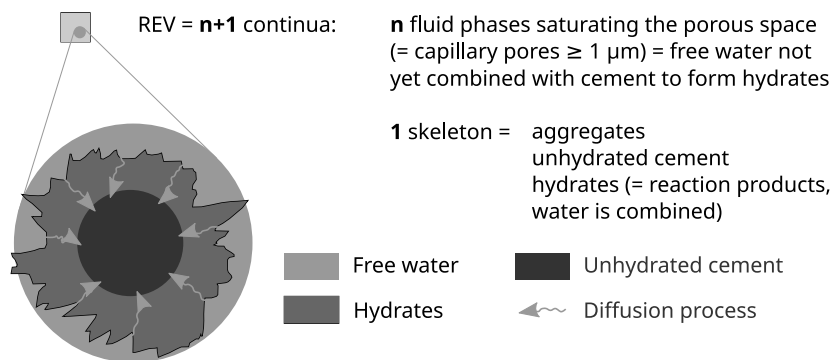


Figure 5.2: Diffusion of water through layers of hydrates (redrawn and modified from [401, Fig. 1, p. 787]).

On the macroscale, a single chemical reaction may represent the hydration process, having the reactant phase at the start and the product phase at the end. On the scale of the REV, because of the chemical reaction, the volume fractions of their constituents (i.e., fluid phases and skeleton) continuously change. Three laws help to describe the energy state¹ and its change within a REV:

- Mass conservation law: "... the total mass is constant during a chemical reaction ..." ([16, p. F68]);
- First law of thermodynamics (= the conservation of energy principle): "... the conservation of energy in all of its forms." ([154, p. 8]); "... energy can be neither created nor destroyed during a process; it can only change forms." ([66, p. 70]); "The internal energy of an isolated system is constant." ([16, p. B10]);

¹Energy exists in different forms ([119, p. 4-3]): gravitational energy, electrical energy, radiant energy, nuclear energy, kinetic energy, mass energy, heat energy, elastic energy, and chemical energy. The last three forms are treated by the approach described here.

- Second law of thermodynamics: "...the quality of energy, i.e. its transformability into efficient mechanical work, can only deteriorate." ([154, p. 9]); "The quantity of energy is always preserved during an actual process (the first law), but the quality is bound to decrease (the second law). This decrease in quality is always accompanied by an increase in entropy." ([66, p. 340]);² "A spontaneous change is accompanied by an increase in the total entropy of the system and its surroundings." ([16, p. B22]).

Note that the REV are assumed to be closed (i.e., sealed conditions). The external mass supply is zero. [154, p. 7ff]

In thermodynamics of continua, so-called state variables (e.g., temperature, or pressure) characterise the state of a system (here, it is the energy state of a REV). By observing those state variables, the REV's state can be determined. An important feature of a state variable is that it "depends only on the current state of the system", but it is independent of "the path or the means of attaining the state". Further, "any change in its value is independent of how the change in state was brought about." Here, the evolution of a state variable of a REV is independent of the variable's evolution rate and of the state of a neighbouring REV (local state postulate). The latter may be not true but is assumed. ([16, p. 273], [133, terms *property* and *state* on p. 58], [154, p. 9])

For each REV, the energy state is described in full by several state variables. Their evolution constitutes the evolution of the REV. The model by [154, p. 9f] considers following state variables:

- ϵ : strain tensor;
- T : absolute temperature;
- m : mass of non-evaporable water per unit volume combined with cement to form hydrates;
- ϵ^p : plastic strain tensor;
- χ : vector of strain hardening and softening variables;
- w : mass of water per unit volume engaged in the micro-diffusion process;
- ϵ^f : flow strain tensor;
- γ : viscous slip.

ϵ and T are external state variables as they are directly measurable on the macroscale.³ The others are internal (hidden) state variables. The latter directly link to physical and chemical dissipative phenomena taking place on the microscale. ϵ^p and χ represent the irreversible skeleton deformation resulting from microcracking (cf. Section 4.6.5 on p. 60). The micro-diffusion process characterised by w results in (viscoelastic) short-term creep (cf. Section 4.6.4 on p. 57). ϵ^f and γ relate to the long-term creep (cf. Section 4.6.4 on p. 57) because of dislocation-like processes in the nanopores of the C-S-H phase (cf. Subsection 'Concept of the microprestress force' on p. 60). And m directly links with strength growth, because m is proportional to the increase in hydrate mass ([401, p. 788]) (cf. Section 4.2 on p. 47). [154, p. 10]

²Entropy is a thermodynamic quantity relating a change in the heat to the temperature. [243, p. 306]

³Normality is assumed for T and ϵ with respect to other state variables. "A particular state variable is said to be normal with respect to a set of state variables if evolutions of this state variable can occur independently from the evolutions of the state variables of the considered set" ([154, p. 11]).

By introducing the intrinsic dissipation⁴ per unit volume of a closed system, a relation between the stress tensor, σ , and the strain tensor, ε , is established (cf. [155, p. 692]). It comprises the Helmholtz energy (or free energy), Ψ , depending on the internal energy, e ,⁵ the internal entropy, S ,⁶ and T (cf. [87, p. 12], [154, p. 9]). Note that Ψ refers to systems where the volume is maintained constant ([16, p. 40]). Now, [154] relates the microscopically phenomena taking place in shotcrete material represented by the internal state variables cited above to energetically conjugated thermodynamic forces using state equations ([212, p. 303]). Then, $\Psi = \Psi(\varepsilon, T, m, \varepsilon^p, \chi, w, \varepsilon^f, \gamma)$ ([154, p. 9]). Thermodynamic forces are ([154, p. 11ff]):

- σ : stresses; driving force of plastic and flow strains;
- A_m : chemical affinity (cf. Section 4.1 on p. 47); driving force of the hydration process;
- A_w : driving force of short-term creep; it's associated with the rate of water diffusion in the capillary pores;
- ζ : hardening force; driving force of strain hardening and softening;
- Γ : microprestress force (cf. Subsection 'Concept of the microprestress force' on p. 60); driving force of viscous slip;
- $\text{grad}T$ ⁷: driving force of the entropy flow.

$\{\sigma; \varepsilon^p; \varepsilon^f; \chi; \gamma; \zeta; \Gamma\}$ determine the intrinsic dissipation, φ_i ; it is $\{A_m; m; A_w; w\}$ for the chemical dissipation, $\varphi_{\rightarrow}$, and it is $\{\text{grad}T; T\}$ for the thermal dissipation related to heat conduction, φ_T . The mechanical dissipation, $\varphi_M = \varphi_i + \varphi_{\rightarrow}$, describes "the rate of mechanical energy transformed irreversibly into heat". $A_m \dot{m}$ ⁸, for example, is a heat source and refers to the energy and heat because of the diffusion of free water through the layers of hydrates (i.e., the latent dissipative heat due to the exothermic nature of the hydration reaction). [154, p. 11f, 17]

Now consider that the thermodynamic forces $\{\sigma; A_m; A_w; \zeta; \Gamma\}$ and the entropy, S , all link to (or are derived from) the free Helmholtz energy, Ψ . Thus, mutual dependencies must exist. They are described via mixed partial derivatives of Ψ . These relations (or dependencies) are termed *Maxwell symmetries* (cf., e.g., [66, p. 649f]) and, here, they constitute thermo-chemo-mechanical couplings in the shotcrete material. Knowing a minimum of material parameters (e.g., from calorimetric or creep tests), those couplings allow for characterising all other state variables (e.g., evolution of the plastic/hardening properties) ([402, p. 1131]). [154, p. 14]

The following subsections give some information on the couplings considered in the shotcrete model, on evolution rates of the microscopic dissipative phenomena (i.e., time derivatives of state variables) mathematically described with evolution equations⁹, and on how some important input parameters can be determined.

5.2.1 Chemo-thermal coupling

When free water combines with cement during hydration (chemical reaction), latent heat is produced and the temperature changes. The coupling links the development of the latent heat to

⁴Intrinsic ... independent of field and boundary conditions (cf. [154, p. 6]).

⁵It is the total energy of a system. [16, p. B13]

⁶Cf. Footnote 2 on p. 72.

⁷*grad* is a mathematical operator that makes a vector from a scalar ([119, p. 14f]). The vector "is in the direction of steepest ascent" and "its magnitude is the rate of increase" "with distance in this direction" [243, p. 15].

⁸Dots on top of letters symbolise derivatives with respect to time.

⁹Evolution equations are used linking the rates of the internal state variables to the corresponding thermodynamic forces ([212, p. 303]). Here also the Maxwell symmetries and resulting couplings are essential (cf. [154, p. 18, 30]).

the energy state and yields the temporal and spatial distribution of the temperature, $T = T(t, x)$, and the hydration degree, $\xi = \xi(t, x)$. The model assumes following heat sources to be negligible and, thus, neglects them: heat due to microcracking and dislocation-like sliding within the hydrates (cf. [399]) and heat due to diffusion of absorbed water leading to short-term creep. Both are dissipative heat effects associated with friction (cf. [87, p. 86]) and mechanical work on the molecular- or microscale because of fluid flow and viscoelastic or plastic deformations. The model also assumes that no significant volume heat sources exist in shotcrete tunnel linings. [154, p. 17f], [158, p. 2104], [402, p. 1125]

The chemical affinity, A_m , depends on the state variables $\{\epsilon; T; \chi; m; w; \gamma\}$. However, as a first approximation, it gets decoupled from $\{\epsilon; T; \chi; w; \gamma\}$ (cf. [401]).¹⁰ Thus, the driving force of the hydration, A_m , depends only on the hydration itself: $A_m \approx A_m(m)$. In this way, the influence of microcracking on strength growth, for example, is neglected.¹¹ [154, p. 24]

Since hydration is thermally activated, the evolution law by [401] used in this model considers the Arrhenius equation. Introducing the hydration degree, $\xi = m/m_\infty$, (cf. Section 4.1 on p. 47) with m_∞ being the asymptotic hydration mass, and with $m_\infty = m_\infty(A_0)$ (cf. [402, p. 1127]), where A_0 is the initial chemical affinity (starting the chemical reaction; [155, p. 694]), a relation between $\dot{\xi}$ and $\tilde{A}$ can be established. $\tilde{A} = A_m/(m_\infty \eta_m)$ is the normalised chemical affinity (cf. [402]). η_m is the viscosity describing the hydration rate. [154, p. 24f]

Knowing the normalised chemical affinity from a laboratory test (cf. Subsection 'Ageing' on p. 75), a two-dimensional, central-symmetric Finite Element simulation then yields the time-dependent temperature and hydration degree field across the lining thickness by solving the heat transfer problem.

For an example of thermo-mechanical simulations considering the same coupling approach, refer to [150].

5.2.2 Thermo-mechanical coupling

This coupling simply yields the strain resulting because of changes in the temperature (i.e., thermal strain). For the concrete material, isotropy of the thermal dilatation is assumed (cf. also Section 4.6.2 on p. 52). [154, p. 15]

5.2.3 Chemo-mechanical coupling

With this coupling, the development of the macroscopic, intrinsic material properties with the hydration degree, ξ , (from the chemo-thermo coupling; cf. first subsection above) is described. It's the Young's modulus, E , the uniaxial peak compressive strength, f_c , the chemical dilatation (i.e., autogenous shrinkage, ϵ^{shr}), the asymptotic long-term creep compliance, J_∞^f , and the characteristic time for the short-term (viscoelastic) creep, τ_w . The model assumes that they all depend on the mass of hydrates formed only (for some of them, weak links to other state variables exist but are neglected; cf. [44, 63, 399, 401] in [154, p. 15f]). The couplings establish the relation between the microscopic phenomena of chemical or mechanical origin and the strains they provoke (cf. [370, p. 285]). For more information on the material properties, refer to Section 4.2 (p. 47) and related subsections in Section 4.6 (p. 51).

¹⁰According to [402, p. 1127] (referring to [401]), the relations between the chemical affinity and the loading history and temperature changes induced by external heat supply are weak. However, in case temperature changes and loading (here in terms of mean stress) are high in magnitude, their influence may be significant.

¹¹Note that the development of strength depends only on m . But the compressive strength at a particular moment depends also on the state of microcracking, χ , (cf. Subsection 'Microcracking—multisurface chemoplasticity' on p. 77).

The model only considers basic creep (in terms of short- and long-term creep; cf. subsections below). It neglects drying creep (cf. Section 4.6.4 on p. 57) on the basis that drying is negligible considering the humid conditions usually present in tunnels ([154, p. 27]). The creep model follows the solidification theory after [32, 31].

The primary model only considers autogenous shrinkage. It neglects drying shrinkage. The software engineers of TUNNEL:Monitor substituted the original formulation with equations suggested by [20] accounting for both autogenous and drying shrinkage. For general information on shrinkage, refer to Section 4.6.3 (p. 54). The information given in the first subsection below refers to the implementation in the primary model.

The following subsections give some information on the microscopic phenomena mentioned above.

Ageing

Predicting ξ requires the knowledge of $\tilde{A}$. In adiabatic¹² or semi-adiabatic calorimetric experiments (at a constant temperature, T ;¹³ semi-adiabatic: experimental device allows for a certain heat diffusion, cf. [154, p. 39]), the accumulated heat is measured that develops during the hardening of a cement paste sample. As the heat release is proportional to $\tilde{A}$, such an experiment yields the development of $\tilde{A}$ with time. Solving the transient heat transfer problem mentioned in Section 5.2.1 results in the development of ξ with time. Fig. 5.3d plots $\tilde{A}$ over ξ . [123, 320] in [172, p. 49], [304, p. 3, 11f]

ξ now can be linked to the consequences of the hardening on the macroscale, the material properties. The evolution of shotcrete strength as a function of ξ and the linearity of the chemical hardening (cf. Fig. 5.3c and Fig. 5.5b) is verified experimentally by, e.g., [44, 63, 263]. ξ_0 in Fig. 5.3c is the so-called percolation threshold (cf. [6]). Below this threshold, the ground-lining system behaves like there would be no lining at all ([156, p. 706]). Beyond the threshold, the material can support a stress deviator and starts to carry load ([370, p. 288]). Here the threshold is 5%. In [154, p. 44], [158, Fig. 2, p. 2106], and [212, Fig. 16, p. 303], it is 1%. However, the threshold can be also higher (cf. [303]). It is assumed that hydration is practically completed after 28 days ([154, p. 50]; cf. also [347, Tab. 1, p. 5]). With a compression test on a shotcrete sample at an age of 28 days, $f_c = f_c(\xi)$ can be described (cf. Fig. 5.5b). [154, p. 30ff]

In a similar way, the Young's modulus, E , and the autogenous shrinkage strains, ε^{shr} , are linked with ξ (cf. Fig. 5.4, and Fig. 5.5a). The model assumes isotropic elasticity, isotropy for the chemical dilatation (i.e., autogenous shrinkage), and a constant Poisson's ratio, ν , (for ν , refer to Section 4.9.1 on p. 67). The slope of the straight line in Fig. 5.4d is characterised by the chemical dilatation coefficient. When evaluating Fig. 5.4, relating graph (a) with (c) and (b) with (d), consider that the relation between ξ and t is nonlinear (cf. [402, p. 1128]). [154, p. 32ff]

Models on basis of the degree of hydration are usually formulated via direct coupling between the release of heat of hydration and strength growth. This works particularly well if the binder does not contain any latent hydraulic components. Now the problem is, in case the concrete mix comprises slag cement or fly ash¹⁴, the coupling between hydration heat and strength growth gets weak. This is because those additives set hydraulically featuring a latent nature ([393] in [353, p. 10]). That means that hydration of these additives proceeds—and contributes to strength growth—even if the temperature and the hydration degree do not change ([355]).

¹²In an adiabatic system or process, no heat energy is transferred to or from the surrounding. However, energy in terms of work may be transferred. [16, p. 268, 309], [243, p. 311]

¹³The temperature, T , influences the kinetics of the hydration. The reaction is faster if T is higher. [401, p. 782]

¹⁴Additives like fly ash and granulated slag are used to improve the properties of the sprayed concrete (e.g., workability, rebound). [289, p. 12]

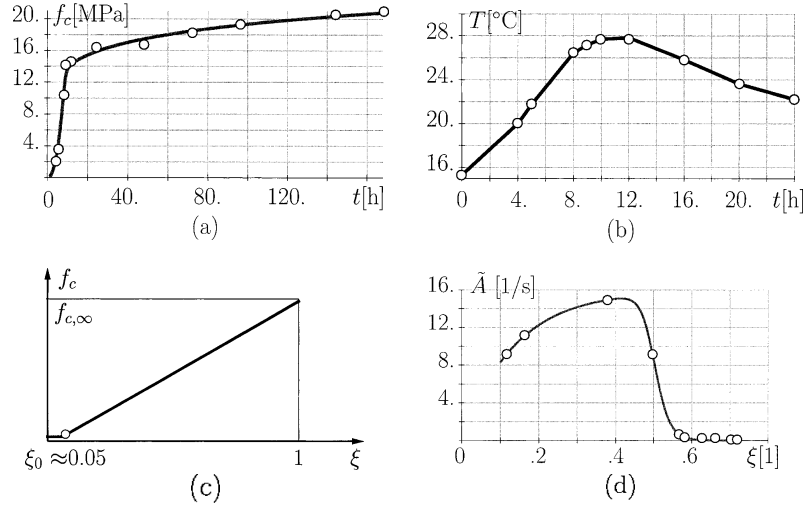


Figure 5.3: Hydration related parameter developments for a shotcrete material (from [155, Fig. 1–3, p. 694f]): (a) uniaxial peak compressive strength, $f_c = f_c(t)$; (b) temperature, T , during hydration; (c) linear chemical hardening, $f_c = f_c(\xi)$; (d) normalised chemical affinity, $\tilde{A} = \tilde{A}(\xi)$. Test data for (a), (b), and (d) from [173]. Note the different scaling of the axis of abscissas in (a) and (b).

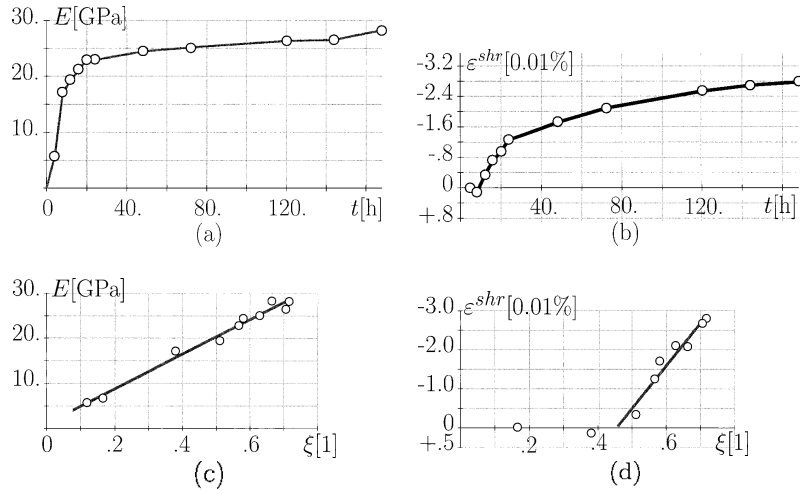


Figure 5.4: Hydration related parameter developments for a shotcrete material (from [155, Fig. 4–5, p. 695]): (a) Young's modulus, $E = E(t)$; (b) autogenous shrinkage strains, $\varepsilon^{shr} = \varepsilon^{shr}(t)$; (c) chemo-mechanical coupling of the Young's modulus, $E = E(\xi)$; (d) chemo-mechanical coupling of the autogenous shrinkage strains, $\varepsilon^{shr} = \varepsilon^{shr}(\xi)$. Test data from [173].

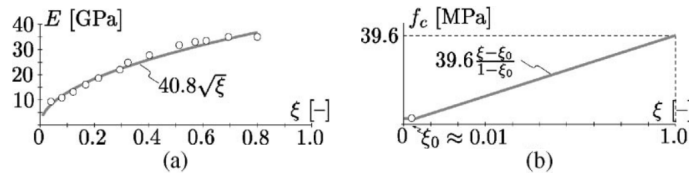


Figure 5.5: Hydration related evolutions of the shotcrete material implemented in the thermo-chemo-mechanical shotcrete model (modified from [211, p. 2363, Fig. 2b–c]): (a) Young's modulus, $E = E(\xi)$; (b) uniaxial peak compressive strength, $f_c = f_c(\xi)$.

Microcracking—multisurface chemoplasticity

The hardening force, ζ , depends on the state variables $\{\chi; m; w; \gamma\}$. ζ relates to the material's strength and, according to experiments, depends only on $\{\chi; m\}$. Thus, the model neglects w and γ (partial decoupling), and chemical hardening (i.e., strength growth) and plastic hardening and softening determine ζ . [154, p. 18]

The model is a two-surface chemoplasticity model comprising a Drucker-Prager¹⁵ loading surface and a tension cut-off (Rankine criterion) (cf. Fig. 5.6; [155, p. 692]). It uses standard multisurface plasticity approaches to determine plastically admissible stress states and for the definition of the flow rule and the hardening rule. Associative hardening plasticity is assumed. [154, p. 19]

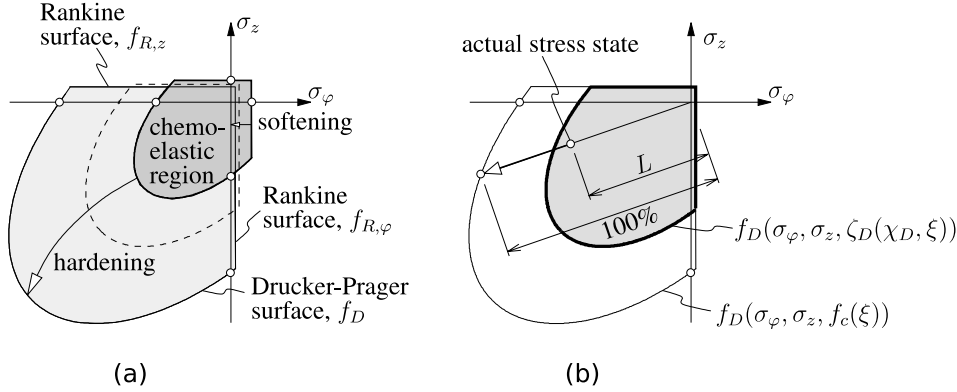


Figure 5.6: Shotcrete model: Yield surfaces in the σ_φ - σ_z stress space (from [210, Fig. 7 on p. 1054, Fig. 22 on p. 1065]): (a) description of the stress space and the yield surfaces; (b) illustration of the level of loading. z is the coordinate in the direction of the tunnel drive. φ is the angular coordinate in the circumferential direction.

The yield functions both on the compression and tension side depend on hardening parameters, namely χ_1 and χ_2 . They make up the vector of strain hardening and softening variables, χ , and are linked to plastic volume dilation, ε_v^p . [154, p. 34f]

All input parameters to calibrate the loading surfaces can be determined from standard laboratory tests ([154, p. 36]):

- uniaxial peak compressive strength, f_c ;
- uniaxial compressive yield strength, f_{cy} ;
- biaxial compressive strength, f_b ;
- uniaxial peak tensile strength, f_t ;
- uniaxial compressive strain at f_c (complete hydration assumed), $\varepsilon_{c,\infty}^u = 0.0022$ m/m (cf. [158, caption of Fig. 1, p. 2105]).

The model assumes isotropy of the chemical hardening: $f_b/f_c = \text{constant}$, $f_{cy}/f_c = \text{constant}$, and $f_t/f_c = \text{constant}$ (cf. Section 4.8 on p. 64). On the compression side, the plastic hardening between $\{f_y; \chi = 0\}$ and $\{f_c; \chi = \chi_u\}$ is described with a quadratic plastic hardening law (cf. Fig. 5.7). R in the graph is the uniaxial chemoplastic compressive threshold. f_{cy} and f_c depend

¹⁵The Drucker-Prager model is acceptable for predominantly biaxial stress states (which is assumed here; cf. Section 4.4 on p. 49) but is insufficient in case of significant triaxial compressive stress states ([260, p. 3135]). In any case, in consequence of the isotropy of chemical and plastic hardening, the model cannot account for a dependence of the friction angle on the hydration degree or on the stresses ([402, p. 1130]).

on ξ . However, the development of strength is still linear and so is the coupling of ξ with χ . Note from Fig. 5.7 that the value of the hardening parameter at f_c , χ_u , is independent from ξ . Note also that plastic hardening is considered being isotropic ([402, p. 1130]). [154, p. 36ff]

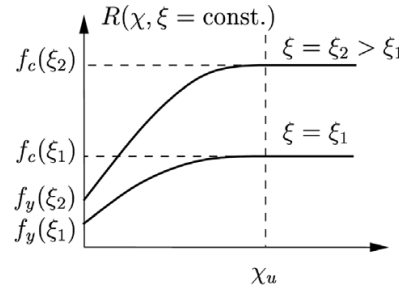


Figure 5.7: Shotcrete model: Quadratic hardening law for two different degrees of hydration, ξ_1 and ξ_2 , (from [158, Fig. 1b, p. 2105]). Note that f_y in the graph is f_{cy} in the text.

On the tension side, the model neglects plastic softening. Thus, in contrast to the compression side, the hardening force, ζ , is determined by ξ only and is independent from χ (i.e., purely chemical hardening assumed). [154, p. 38]

Dislocation-like sliding in hydrates—long-term creep

The microprestress force, Γ , depends on the state variables $\{\chi; m; w; \gamma\}$. However, based on experimental results, it gets decoupled from χ and w . The model further neglects the influence of the evolution rate of m on $\dot{\Gamma}$. [154, p. 20f, 49]

Long-term creep starts with reaching the percolation threshold, ξ_0 , (cf. p. 75). The material then can bear a deviatoric stress state. [370, p. 288]

The flow strains, $\dot{\epsilon}^f$, and the viscous slip, $\dot{\gamma}$, are activated thermally (cf. [27]). Thus, both the evolution law for $\dot{\epsilon}^f$ and the evolution law for $\dot{\gamma}$ consider the Arrhenius equation (cf. [16, p. 640]). [154, p. 21f]

Standard creep tests on concrete (or shotcrete) samples can be used to determine the required material parameters, but

- the conditions should be isothermal;
- the applied constant stress should be below $0.3f_c$ (i.e., below the limit of proportionality; cf. Section 4.9 on p. 66);
- the applied stress should not vary during the loading phase of the test but remain constant;
- shrinkage tests (without loading) should be carried out simultaneously under the same conditions to get the shrinkage strains;
- several creep tests should be carried out varying the age of loading and the age of unloading (for the reversible part); this allows to determine the effect of ageing on creep.

With the constant compressive loading, σ , and the measured strain changing with time, ϵ , the compliance, $J = \epsilon/\sigma$, can be calculated (cf. Fig. 5.8a). The latter "is the strain in the direction of the load application per unit of (uniaxial) stress." It's a characteristic material property ([153, p. 9]). Note that for the determination of the creep parameters, shrinkage strains and thermal strains must be subtracted out of ϵ . [154, p. 23, 46ff]

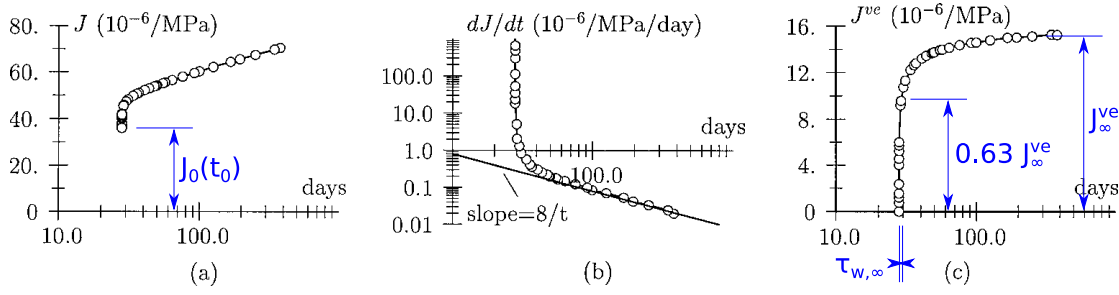


Figure 5.8: Exemplary compliance for a shotcrete using data from a creep test by [173] (modified from [370, Fig. 2a–c, p. 288]; modifications in blue). Development of the total mechanical compliance, J , (a), the rate of J (b), and the short-term creep compliance, J^{ve} , (c) with time. Note the logarithmic scaling of the axis of ordinates in all graphs, and of the axis of abscissas in graph (b). Note that J^{ve} in graph (c) is J^v in the text.

This compliance, or the stress-strain relation from a creep test in general, covers three components (modified after [154, Eq. 3.46, p. 49]):

$$\int dJ = \int_{t_0}^t \frac{dJ}{dt} dt = J(t, t_0) = J^v(t, t_0) + J^f(t, t_0) + J_0(t_0). \quad (5.1)$$

The constant of integration, $J_0(t_0) = 1/E(\xi(t_0))$, is the instantaneous (or elastic) compliance at the moment of load application, t_0 . $J^v(t, t_0)$ is the short-term creep compliance. And $J^f(t, t_0)$ is the long-term creep compliance. Because Eq. 5.1 lacks of components relating to (stress-free) shrinkage or thermal strains, it is termed total mechanical compliance. Fig 5.8b graphs the rate of total compliance, dJ/dt . Note here that the curve tends towards a straight line. According to experiments by [214] (cf. Fig. 1b in [370, p. 287]) and [400], all dJ/dt curves from tests on shotcrete samples different in their ages at the moment of loading, t_0 , tend towards such a straight line.¹⁶ [154, p. 49f], [370, p. 288f]

Diffusion of absorbed water—short-term creep

The thermodynamic force determining short-term creep, A_w , depends on the state variables $\{\epsilon; T; \chi; m; w; \gamma\}$. The model assumes the influence of $\{T; \chi; \gamma\}$ insignificant and neglects it. Next to the stiffness, the stress, and a function of m , the evolution law for the viscoelastic creep strain, $\dot{\epsilon}^v$, depends on τ_w . It's the characteristic time¹⁷ of the micro-diffusion process that results in short-term creep. $\tau_w = \tau_w(m)$ and, thus, it depends on the shotcrete age. This accords with statements by [358]. [154, p. 22f], [370, p. 285]

Similar as for the long-term creep (cf. previous subsection), the compliance is introduced. It simplifies the evolution law assuming that the asymptotic short-term (viscoelastic) creep compliance, J_∞^v , is independent from m (cf. [370]). [154, p. 23]

Now, subtracting the elastic compliance, $J_0(t_0)$, and the long-term compliance, $J^f(t, t_0)$, from the total mechanical compliance, $J(t, t_0)$, the short-term compliance, $J^v(t, t_0)$, results (cf. Eq. 5.1). Fig 5.8c gives an example. Here, J_∞^v results to 15×10^{-6} MPa. [370] introduces the linear relation $\tau_w = \xi \cdot \tau_{w, \infty}$ and assumes $\tau_{w, 28} \approx \tau_{w, \infty} \approx \text{constant}$. So, ageing has no influence on the short-term creep characteristics the moment the shotcrete is 28 days in age or older.

¹⁶However, the asymptotic value of J depends on t_0 and on the temperature, T . The earlier the concrete is loaded and the higher T , the higher is J . (cf. [370, p. 288f] and Section 4.6.4 on p. 57)

¹⁷This time parameter is also termed retardation time and is a characteristic material property. Its general form is $\tau = \eta/E$. It describes the time required for a process to reach a quasi-static state. After that (i.e., $(t - t_0) > \tau$), $\dot{J} \approx 0$. [138] in [153, p. 29]

The non-ageing compliance for the short-term creep is ([370, Eq. 33, p. 288])

$$J^v = J_\infty^v \left[1 - \exp \left(-\frac{t - t_0}{\tau_w} \right) \right]. \quad (5.2)$$

With $t - t_0 = \tau_w = \tau_{w,\infty}$, Eq. 5.2 yields the share of the short-term creep compliance at the moment after which ageing effects are assumed to be insignificant:

$$J^v = J_\infty^v [1 - \exp(-1)] = 0.63J_\infty^v. \quad (5.3)$$

Evaluating $0.63J_\infty^v$ in Fig 5.8c allows to determine $\tau_{w,\infty}$ from the same graph. Here, $\tau_{w,\infty} \approx 1$ day. [154, p. 49f], [370, p. 289]

By performing several creep tests, one can approximate the development of J_∞^v and $\tau_{w,\infty}$ with ξ . For the former, [304], for example, consider a negative linear correlation: $J_\infty^v(\xi) = J_\infty^{v,ini}(1 - \xi)$.

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